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# **Oskarshamn site investigation**

**Hydraulic interference and tracer testing of a rock-soil aquifer system between HLX35 and HLX34, SSM000037, SSM000222 and SSM000223**

## **Subarea Laxemar**

Mansueto Morosini, Svensk Kärnbränslehantering AB

Eva Wass, Geosigma AB

March 2007

#### **Svensk Kärnbränslehantering AB**

Swedish Nuclear Fuel and Waste Management Co Box 5864 SE-102 40 Stockholm Sweden Tel 08-459 84 00 +46 8 459 84 00 Fax 08-661 57 19 +46 8 661 57 19



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*Keywords:* Hydrogeology, Groundwater, Pumping test, Tracer test, Dilution measurement, Transmissivity, Storage coefficient, Soil aquifer, Rock aquifer.

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# **Abstract**

A field investigation is undertaken in order to study the hydraulic contact between rock and soil aquifer. It is investigated by means of pressure interference and tracer testing. A borehole drilled into a deformation zone was pumped while three soil wells and one borehole were used for response observations. Tracer testing was done with conservative tracers only, in two steps. Firstly, dilution measurements were done in the soil wells under unstressed conditions in order to obtain natural groundwater flow velocities and secondly a forced gradient test was done where tracer was injected in one of the soil wells and recovered in the pumped borehole. A hydraulic contact was confirmed between rock and soil in one of the wells. Aquifer parameters, mainly transmissivity and storage coefficient, were calculated for rock and for soil aquifer. Solute transport parameters were quantified.

# **Sammanfattning**

En fältundersökning har genomförts i syfte att studera den hydrauliska kontakten mellan jordakvifären och bergakvifären. Undersökningen har utförts som ett kombinerat interferenstest och spårförsök.

Ett borrhål som penetrerar en deformationszon har pumpats samtidigt som eventuella responser har observerats i tre jordrör och ett annat borrhål i berg som också penetrerar nämnda deformationszon. Spårförsöket genomfördes i två steg där endast konservativa spårämnen användes. I ett första steg utfördes utspädningsmätningar i jordrören för att bestämma grundvattnets naturliga flödeshastighet. I nästa steg utfördes ett spårförsök mellan borrhålet i berget och ett av jordrören. Härvid pumpades berghålet samtidigt som ett konservativt spårämne injicerades i jordröret. Vattenprover togs från berghålet för analys av spårämnet. En hydraulisk kontakt kunde konstateras mellan jord och berg. Såväl diffusivt som advektivt samt transportparametrar kunde kvantifieras. Akvifärparametrarnas transmissivitet och magasinkoefficient kunde också kvantifieras för såväl jordakvifär som bergakvifären.

# **Contents**



# <span id="page-5-0"></span>**1 Introduction**

This document reports the results obtained when performing an interference- and tracer test using borehole HLX35 as pumping hole and borehole HLX34 and SSM000037, SSM000222 and SSM000223 as observation wells. This is one of the activities performed within the site investigation at Oskarshamn. The work was carried out in accordance with activity plan AP PS 400-05-108.

The field activities were performed during 2006-01-10 to 2006-03-06.

In Table 1-1 controlling documents for performing this activity are listed. Both activity plan and method descriptions are SKB's internal controlling documents.

The original results from this avtivity are stored in the primary data bases (SICADA and/or GIS) and that they are traceable through by the activity plan number.

The investigation area is shown in Figure 1-1 and Appendix 8 while information of the tested boreholes is in Appendix 3.



*Figure 1-1. Overview over the test area. The location of borehole HLX34 in relation to the above is shown in Figure 1-3. A general site investigation map showing the location of HLX35 is in Appendix 8.* 

<span id="page-6-0"></span>



## **1.1 Hydrogeological setting**

#### **1.1.1 Lithology and stratigraphy**

The investigation area is situated in Ävrö granite (granite to quartzmonozodiorite), generally porphyritic. Soil aquifer is comprised by a thin 4–7 m till layer overlaying the Ävrö granite bedrock. A deformation zone (NS059A) has been identified in the bedrock whose north-south extension coincides with the small stream Ekerumsån, see Figure 1-1. Three soil wells have been drilled with a 1 m screen making up the contact to the aquifer. In SSM000037 and SSM000222 the screen is at the rock/till contact while in SSM000223 the filter is situated in the till.

Soil wells SSM000222 and SSM000037 are situated in the near proximity  $(1-2 m)$  of Ekerumsån while SSM000223 is about 30 m east of the stream, Figure 1-1.

#### **1.1.2 Water levels**

Rest groundwater levels are about 1.3 m below ground in SSM000037 and SSM000222 in the sand and till layer respectively. In SSM000223 the level rests a about 2.4 m, in sandy till layer. Hence these aquifers maybe classified as unconfined, Figure 1-2. These wells are situated in topographically low in catchment 9:3 with a size of  $0.222 \text{ km}^2$ .



*Figure 1-2. Stratigraphy and restwater levels in the soil wells.*

<span id="page-7-0"></span>The catchment consist of 79% coniferous an mixed forest, 17% cultivated agricultural land and 3% open land. The maximum difference in elevation is 19 m. The aquifer where the soil wells are situated is believed to be recharged from the higher surrounding terrain to the west and the east, Figure 1-3. The initial heads, prior to pumping, are 3 m higher in the rock aquifer than in the soil, Appendix 2.

Further it appears that the head in the stream is slightly higher than in the soil aquifer. It is estimated that the head values in the stream are accurate within  $\pm 0.1$  m due to uncertainties in positioning the pressure gauge.

#### **1.1.3 Deformation zone**

Boreholes HLX34 and HLX35 were drilled through deformation zone NS059A. The zone as conceptualized in Appendix 2 was inferred from four independent data sources; airborne geophysics /SKB 2006/ based on /Triumf 2004/, ground geophysics /Thunehed et al. 2004/, refraction seismics /Lindqvist 2004/ and drilling /Sigurdsson et al. 2005/. The deformation zone is trending north-south and dipping slightly towards west.

Soil well SSM000223 is apparently drilled at a point in the rock with low seismic velocity which is interpreted as being coincident to deformation zone NS059A. Water strike in borehole HLX35 observed during drilling were at borehole lengths 98.5 m, 125 m and 142 m /Sigurdsson et al. 2005/ and also during flowlogging at borehole lenghts 124 m and 133 m /Rohs 2006/. It is hence conjectured that these may be linked between with the low seismic velocity points, thus allowing to calculate a dip for NS059A of 71° west.



*Figure 1-3. Catchment 9:3 of tributary to Ekerumsån and topography.* 

# <span id="page-8-0"></span>**2 Objective and scope**

The objective with the undertaken tests is to investigate the nature of the hydraulic contact between the soil aquifer and the rock aquifer, if any and also to parameterise aquifer- and flowpath properties. Can a hydraulic contact be observed and what is the response for the given hydrogeological setting? This is warranted as a mean to provide the regional modelling exercise with a basis for conceptual understanding and parameterisation of aquifer properties.

The scope of work comprised the following main components:

- Tracer dilution measurements in all three soil wells under natural hydraulic gradient.
- Induce a hydraulic gradient in the soil aquifer by pumping in HLX35.
- Tracer dilution measurements in all three soil wells under forced hydraulic gradient.
- Pressure interference test between bedrock– soil aquifer with HLX35 as pumping borehole and HLX34, SSM000037, SSM000222 and SSM000223 as observation wells.
- Tracer test between HLX35 and SSM000222 and SSM000223 with the purpose to obtain a breakthrough curve at HLX35 and related parameters.

# <span id="page-9-0"></span>**3 Equipment**

## **3.1 Description of equipment/interpretation tools**

### **3.1.1 Hydraulic interference test**

The pumping and interference test was performed with an integrated field unit consisting of a container at HLX35 housing a

- submersible pump: Grundfoss SPE5-70, range is about 5–100 L/min,
- absolute pressure transducer: Druck PTX1830, 10 bar range and  $\pm$  0.1% accuracy,
- water level dipper,
- flow gauge: Krohne IFM1010 electromagnetic, 0–150 L/min.



*Figure 3-1. Container housing the testing equipment (right) and instrumentation inside (left) in borehole HLX35.* 

<span id="page-10-0"></span>The observation wells were equipped with absolute pressure gauges datalogger as follows:

- SSM000037: 2 bar Druck PTX-1830 gauge 3.7 m b toc with accuracy of  $\pm$  0.1% of full scale and Mitec logger
- SSM000222: 30 PSIA LevelTroll integrated gauge and logger 5.0 m b toc with accuracy of  $\pm$  0.2% of full scale and resolution of  $\pm$  0.01% of full scale
- SSM000223: 30 PSIA LevelTroll integrated gauge and logger 8.0 m b toc with accuracy of  $\pm$  0.2% of full scale and resolution of  $\pm$  0.01% of full scale
- HLX34: 30 PSIA MiniTroll integrated gauge and logger 10.0 m b toc along the borehole with accuracy of  $\pm$  0.2% of full scale and resolution of  $\pm$  0.01% of full scale

All pressure gauges were set to log data every 10 seconds during the test.

All gauges are calibrated from the factory. During the test the pressure gauge reading are compared to those from a water level dipper for the purpose of checking sensibility of readout.

Additionally, the Druck pressure gauge was calibrated in September 2005 against different calibration pressures.

The flow gauge was checked against bucket of known volume and stopwatch in April 2005.

#### **3.1.2 Tracer test**

The tracer dilution tests and the tracer tests were performed using three identical equipment set-ups, i.e. allowing three sections to be measured simultaneously. A schematic drawing of the tracer test equipment is shown in Figure 3-2. The basic idea is to have an internal circulation in the borehole section. The circulation makes it possible to obtain a homogeneous tracer concentration in the borehole section and to sample the tracer concentration outside the borehole in order to monitor the dilution of the tracer with time.

Circulation is controlled by a down-hole pump with variable speed and measured by a flow meter. Tracer injections are made with a peristaltic pump and sampling is made by continuously extracting a small volume of water from the system through another peristaltic pump (constant leak) to a fractional sampler. The equipment and test procedure is described in detail in SKB MD 368.010, SKB internal document.

In the withdrawal borehole two different equipments were used for sampling. Samples from the outgoing pumped water were taken by an automatic programmable 24-valve sampler, producing discrete 1 litre samples, Figure 3-3. A tube sampler was used for manual sampling at three different depths (94 m, 110 m and 130 m) in the borehole, Figure 3-4. The external and internal diameter of the tube is 6 and 4 mm, respectively. The water content in the tube constitutes one sample, and the volume of each sample was about 100 ml. At the lower end of the tube, a weight is added in order to stretch the tube and thereby prevent fastening.

The tracers used were a fluorescent dye tracer, Uranine (Sodium Fluorescein) from Merck (purum quality) and Rhodamine WT from Holiday Dyes Inc. (techn. quality).



*Figure 3-2. Schematic drawing of the equipment used in tracer dilution measurements.*



*Figure 3-3. The automatic programmable sampler; magnetic valves (left) and control unit (right).*



*Figure 3-4. Schematic picture of the tube sampling equipment (left), hose reel (right, above) and lower end of the hose inserted in the PEM-tube at the borehole casing (right, below).*

# <span id="page-13-0"></span>**4 Execution**

## **4.1 General**

The testing was executed as a combined pressure interference- and tracer test including the following main tasks:

- Tracer dilution measurements in all three soil wells under natural hydraulic gradient.
- Induce a hydraulic gradient in the soil aquifer by pumping in HLX35.
- Tracer dilution measurements in all three soil wells under forced hydraulic gradient.
- Constant rate pressure interference test between bedrock– soil aquifer with HLX35 as pumping borehole and HLX34, SSM000037, SSM000222 and SSM000223 as observation wells.
- Tracer test between HLX35 and SSM000222 and SSM000223 with the purpose to obtain a breakthrough curve at HLX35 and related parameters.
- Measurement of groundwater head recovery upon pumpstop.

The water level in the stream Ekerumsån was also measured.

In order to measure the water dynamics and transport properties of the aquifer, tracer injections were made in soil wells nearby the pumping borehole HLX35 during the interference test performed. Initially, before the pumping started, tracer dilution tests were performed in the three soil wells, SSM037, SSM000222 and SSM000223, for measurement of the groundwater flow. After three days of pumping, tracers were injected in two of the soil wells, SSM000222 and SSM000223, and the pumping borehole HLX35 was continuously sampled for tracer breakthrough.

The method descriptions used were "System för hydrologisk och meteorologisk datainsamling. Vattenprovtagning och utspädningsmätning i observationshål." (SKB MD 368.010, SKB internal document) and "Metodbeskrivning för flerhålsspårförsök" (SKB MD 530.006, SKB internal document).

## **4.2 Preparations**

The pumpingtest equipment was calibrated according to 3.1.1 and data loggers were set to log data every 10 seconds .

The preparations included mixing of the tracer stock solution, functionality checks of the equipment and calibration of the peristaltic pumps used for sampling and tracer injections.

## **4.3 Execution of field work**

Pumping was performed in HLX35 with a withdrawal rate of 104 L/min and samples were taken and analysed for tracer breakthrough. Test perfomed are compiled in Table 4-1.





\* 1B: Pumping test-submersible pump. 2: Interference test.

Water samples were taken on January 16<sup>th</sup> (sample #: 10765) and 18<sup>th</sup> (sample #: 10766) and submitted for class five analysis to the Äspö laboratory. Results from sampling on 18<sup>th</sup> January are compiled in Table 4-2. A complete record of the analytical results is given in Appendix 8.

Regular measurements of the electrical conductivity of the pumped water were done with the purpose of environmental control. No discharge water with electrical conductivity above 300 mS/m is allowed to be discharged to the ground, it should go to the sea. These measurements showed an increase from 90 mS/m at pumpstart to 150 mS/m at pumpstop. The electric conductivity in the soil aquifer is about 50 mS/m.

For measurements of the groundwater flow, tracer dilution tests were performed in boreholes SSM000037, SSM000222 and SSM000223 both before and during pumping in HLX35.

The tests were made by injecting a slug of tracer in the selected borehole sections and allowing the natural groundwater flow to dilute the tracer. All three sections were injected simultaneously. The tracer solution was continuously circulated and sampled using the equipment described above. After four days the pumping was started in HLX35 and the dilution tests continued for another three days.



#### **Table 4-2. Major ions in water sample from HLX35.**

<span id="page-15-0"></span>Boreholes SSM000222 and SSM000223 were chosen for tracer injections. In both boreholes tracer was injected through an "exchange" procedure, i.e. water was also withdrawn from the section during the tracer injection, the same volume as added through the injection. The tracer injections were made as decaying pulse injections, i.e. injection of a tracer pulse in a circulating system without excess pressure. A simple and reasonable assumption is that the amount of tracer that leaves the injection section (and into the transport path) is proportional to the tracer concentration in the injection section. Samples were continuously withdrawn from the injection sections to monitor the tracer injection versus time.

The samples were analysed for dye tracer content at the Geosigma Laboratory using a Jasco FP 777 Spectrofluorometer.

## **4.4 Data handling/post processing**

#### **4.4.1 Hydraulic interference test**

Data from all pressure gauges was corrected with respect to atmospheric pressure and converted to groundwater head expressed in metre above sea level. The gauges were from observation wells HLX34, SSM000037, SSM000222, SSM000223 from HLX35 and the stream Ekerumsån. A water density on  $1,000 \text{ kg/m}^3$  was used.

## **4.5 Analyses and interpretations**

- Give a brief description of the method of analysis and the underlying theories, with references to the literature and which predictions that are made.
- Describe how the interpretation tool is validated.
- Generally, interpretation should only include data from this specific activity and not data from other activities. Also, no interpretation should be done that expresses influence on the deep repository.

#### **4.5.1 Hydraulic pumping- and interference test**

Level data from boreholes, wells, stream and the precipitation were plotted as linear timeseries to assess cofluctuations, if any, and deduce cause/effects processes, see Appendix 1 and 4.

Analysis of pumping and interference test was then done according to method descriptions SKB MD 321.003 v.1 (Metodbeskrivning för hydrauliska enhålstester), SKB MD 330.003 v.1 (Metodbeskrivning för interferenstester) and instruction SKB MD 320.004e v.1 (Instruction for analysis of injection and single hole pumping tests). These SKB internal controlling documents.

Briefly, the analysis is based on diagnostic log-log plot of the drawdown and the derivative of the drawdown in order to understand the different evolving flow regimes during the test. This is utilized to chose an appropriate analytical model from which the aquifers parameters, transmissivity and storage coefficient, are calculated. Observation wells were modeled with the line source solution while for the pumping well the borehole volume was included .

Interference tests between HLX35–SM000223 and HLX35–HLX34 were interpreted for response index, flow regime and aquifer parameter. Of the soil wells only the responses in SSM000223 were partly caused by the pumping in HLX35. SSM000037 and SSM000222 were consequently not possible to analyse.

The test interpretation for flow regimes and aquifer parameters was done with Ecrin v4.02.03 software from Kappa Engineering (France).

<span id="page-16-0"></span>Response index follow SKB MD 330.003,

Index 1:

 $r_s^2/dt_L$  = normalised distance  $r_s$  with respect to the response time [m<sup>2</sup>/s].

Index 2:

 $s_p/Q_p$  = normalised drawdown with respect to the pumping rate [s/m<sup>2</sup>].

Additionally, a third index was calculated including drawdown and distance. This index is calculated as follows:

Index 2 new:



The classification based on the indices is given as follows:





The response indexes which were calculated are compiled in Table 5-2.

#### **4.5.2 Tracer test**

#### *Tracer dilution tests*

Flow rates were calculated from the decay of tracer concentration versus time through dilution with natural unlabelled groundwater, cf. /Gustafsson 2002/. The so-called "dilution curves" were plotted as the natural logarithm of concentration versus time. Theoretically, a straight-line relationship exists between the natural logarithm of the relative tracer concentration  $(c/c_0)$  and time, *t* (s):

$$
\ln\left(c/c_0\right) = -\left(\mathcal{Q}_{bh}/V\right) \cdot \Delta t \tag{4-1}
$$

where  $Q_{bh}$  (m<sup>3</sup>/s) is the groundwater flow rate through the borehole section and  $V$  (m<sup>3</sup>) is the volume of the borehole section. By plotting  $ln(c/c_0)$  versus *t*, and by knowing the borehole volume *V*,  $Q_{bh}$  may then be obtained from the straight-line slope. If  $c_0$  is constant, it is sufficient to use ln *c* in the plot.

The sampling procedure with a constant flow of 4–6 mL/h also creates a dilution of tracer. The sampling flow rate is therefore subtracted from the value obtained from Equation 4-1.

#### <span id="page-17-0"></span>*Tracer tests*

Tracer mass recovery was calculated for the flow path SSM000223  $\rightarrow$  HLX35. Before the injection a sample of the stock solution was taken and the tracer concentration of the sample was measured. The injected volume together with the tracer concentration of the stock solution was used to determine the injected mass. The tracer mass recovered in the pumping borehole section was determined by integration of the breakthrough curves for mass flux (mg/h) versus time (h).

The evaluation of the tracer test has also involved computer modelling using a simple onedimensional advection-dispersion model /Van Genuchten and Alves 1982/. From the computer modelling, dispersivity and mean travel times were determined using an automated parameter estimation program, PAREST /Nordqvist 1994/. PAREST uses a non-linear least square regression where regression statistics (correlation, standard errors and correlation between parameters) also is obtained.

The chosen one-dimensional model assumes a constant fluid velocity and negligible transverse dispersion, cf. Equation 4-2.

$$
\partial C/\partial t = D(\partial^2 C/\partial x^2) - v \cdot \partial C/\partial x \tag{4-2}
$$

where:  $D =$  Dispersion coefficient

 $v =$  fluid velocity (m/s)

*C* = concentration of solute

 $x =$  distance from injection point (m)

 $t =$  time (s)

According to /Ogata and Banks 1961/ and /Zuber 1974/, the dispersion in a radially converging flow field can be calculated with good approximation by equations valid for one-dimensional flow. Although a linear flow model (constant velocity) is used for a converging flow field, it can be demonstrated that breakthrough curves and parameter estimates are similar for Peclet numbers of about 10 and higher.

/Van Genuchten 1982/ gives a solution for step input with dispersion over the injection boundary. The solution of Equation 4-2 then is:

$$
C/C_o = \frac{1}{2} \operatorname{erfc} \left[ (x-v \cdot t) / Z \right] + (V/\pi)^{1/2} \exp \left[ (x-v \cdot t)^2 / (4D \cdot t) \right]
$$
  
\n
$$
\frac{1}{2} \left[ 1 + v \cdot x/D + V \right] \exp \left[ v \cdot x/D \right] \operatorname{erfc} \left[ (x+v \cdot t) / Z \right]
$$
  
\nwhere:  $Z = 2(D \cdot t)^{1/2}$   
\n
$$
V = v^2 t/D
$$
 (4-3)

Variable injection schemes were simulated by superposition of the solution given in Equation 4-3.

The fit of the breakthrough curves using a three-parameter fit included velocity, *v*, dispersion coefficient, *D*, and the so called F-factor which corresponds to injected mass divided by fracture volume, *Minj*/*Vf*.

#### **4.6 Nonconformities**

Part of the groundwater level data during recovery in HLX35 was lost for the period 15–20 February 2006. Accidentally the data was erased from the logger when retrieving it to the computer.

# <span id="page-18-0"></span>**5 Results**

The original results are stored in the primary data bases (SICADA) which is utilised for further interpretation (modelling). The data is traceable in SICADA by the Activity Plan number (AP PS 400-05-108) and the Field Note numbers 978 (pumping- and interference tests) and 1,250 (tracer tests).

The original results are stored in the primary data base SICADA. These data is available for further interpretation and is traceable by the Activity Plan number (AP PS 400-05-108).

## **5.1 Hydraulic pumping- and interference test**

#### **5.1.1 Hydrograph responses**

Linear response plots are presented in the Appendices. Appendix 1 shows the coplotting between the hydrographs the pumped borehole HLX35 and the observations wells in soil SSM000037, SSM000222 and SSM000223. Appendix 4 shows the holes colpotted with the precipitation events as measured at the Äspö meteorological station. Appendix 6b shows the response hydrograph of borehole HLX34, situated in the north-south trending deformations zone NS059A. It is evident that HLX35 and 34 exhibit large drawdowns of 11.0 m and 4.3 m respectively, caused by the pumping in HLX35. These are amenable to standard pumping tests evaluations for aquifer parameters and response indexes.

Whereas the responses in the soil wells are clearly influenced by other sources of disturbance. The water level in the soil wells varies between 0.2–0.3 m over the testperiod while the level in Ekerumsån on fluctuates within 0.1 m. There is however a striking resemblance in the pattern of variation amongst all soil wells and the stream. These show an ongoing declining level by the time pumping in HLX35 starts. Attempt to correlate the waterlevel fluctuations in the stream and soil aquifer show a good correspondence with precipitation events, Appendix 4. There are three main precipitation events centered at 21<sup>st</sup> January, 7<sup>th</sup> February and 19<sup>th</sup> February respectivelly. These events all correspond to rising and subsequent decline in water level in the soil wells and the stream.

It is hence believed that precipitation to a large extent controll these waterlevel fluctuations.

#### **5.1.2 Response indexes**

A constant flow rate was kept in HLX35 at 104 L/min, actually it increased slightly from are 103.8 to 104.7 L/min. Drawdown and response time for all boreholes are shown in Table 5-1. Point of application in the table is the point in the borehole which is believed to have most of the concta with the observation borehole. Usually this is the main inflow point in the hole. For HLX34 it was calculated as the shortest distance along the presumed deformation zone. For SSM000223, several conceptually different transport paths are plausible, a priori:

- a) from soil wells to borehole through the soil and down along the borehole,
- b) shortest straight line distance between point of application, and
- c) between point of applications through tourtous path controlled by the fracturing in the granite and the soil/rock contact.



<span id="page-19-0"></span>**Table 5-1. Measured test sections and selected parameters.**

<sup>1)</sup> Ground distance.

2) Between inflow points.

3) Along soil/rock contact to deformation zone NS059A.

The choice of most representative transport path is based on consideration of 1) the conceptual hydrogeological setting presented in Appendix 2 and discussed in Section 1.1, and 2) of the fact that there were only hydraulic responses in SSM000223 and not SSM000037 or SSM000222. These two (SSM000037 and SSM000222) have a stronger covariation with the waterlevel in Ekerumsbäcken than SSM000223 due to its proximity to the stream. Probably this influence of the stream is masking any small influence from the pumping of HLX35. With this in mind option c) above is adopted as most representative distance of 132 m for responses between HLX35 and SSM000223.

#### **5.1.3 Aquifer parameters**

As explained in Section 5.1.1, soil wells SSM000037 and SSM000222 do not appear to respond to the pumping in HLX35 at all while observation holes HLX34 and SSM000223 show a clear response at the onset of pumping and at pumpstop. The later also show level fluctuations controlled by other factors than pumping. Hence, aquifer parameters are only calculated for HLX34, HLX35 and SSM000223. Observation boreholes HLX34 and SSM000223 were analysed first in order to calculate a storage coefficient (S) which could be utilised as input to the analysis of the pumped borehole HLX35. The distance between point of application for the pumped and observation holes utilised for the calculation of the storage coefficient (S) is 171 m for HLX34 and 132 m for SSM000223, see Section 5.1.2 and Table 5-1.

Below is a qualitative discussion of the main points in the performed analysis for each borehole. Derived aquifer parameters are compiled in Table 5-2. In Appendix 6 there is a full presentation input data, diagnostic log-log plot, semi-log plot and history plot, all with measured data and simulated responses

#### *SSM000223*

Due to the large disturbances on the waterlevel induces by precipitation only the drawdown phase was amenable for analysis. Furthermore for the simulations only the first part of the drawdown data , from start of pumping to first precipitation event, was utilised to simulate against. Diagnostic log-log plot show a radial flow regime which could be fitted reasonably well with the analytical Theis model. Derived aquifer parameters are given in Table 5-2.

<b>Parameter</b>	Unit	HLX35 pump bh	HLX34 obs bh	SSM000223 observation well				
<b>Test parameters</b>								
Secup	[m b toc]	6.1	9.1	6				
Seclow	[m b toc]	151.8	151.8	8				
L	[m]	145	142.7	$\overline{2}$				
$Q_p$	[L/min]	104	0	0				
$Q_p$	[m <sup>3</sup> /s]	$1.7333 \cdot 10^{-3}$	0	0				
$dh_{p}$	[m]	11	4.27	n/a due to external disturbances				
$t_{p}$	[min]	43,206	43,206	43,206				
tF	[min]	22,877	15,678	15,677				
$\mathsf{h}_0$	[m.a.s.l.]	13.89	14.23	10.97				
$h_i$	[m.a.s.l.]	14.04	14.26	10.95				
$h_{p}$	[m.a.s.l.]	3.36	9.95	n/a due to external disturbances				
$h_f$	[m.a.s.l.]	14.27	14.48	n/a due to external disturbances				
Calculated parameters from the drawdown phase								
Flow regime	$[ - ]$	Double porosity	Double porosity	Radial (IARF)				
C	$m^3$ /pa	$3.6 \cdot 10^{-6}$	Line source	Line source				
ξ	$[ - ]$	$-5.8$						
T	$[m^2/s]$	$1.1 \cdot 10^{-4}$	$1.0 - 10^{-4}$	$3.9.10^{-3}$				
Κ	[m/s]	$7.7 \cdot 10^{-7}$	$6.6 \cdot 10^{-7}$	$2.0 \cdot 10^{-3}$				
S	$[ - ]$	$\overline{\phantom{0}}$	$1.5 \cdot 10^{-4}$	$8.1 \cdot 10^{-3}$				
ω	$[ - ]$	0.08	0.5					
λ	$[ - ]$	$8.0 \cdot 10^{-7}$	$1.5 \cdot 10^{-6}$					
Calculated parameters from the recovery phase								
Flow regime	$[ - ]$	Double porosity	Double porosity	Analysis n/a due to external disturbance				
$\mathsf C$	$m^3$ /pa	$5.6 \cdot 10^{-7}$	Line source					
ξ	$[ - ]$	$-5.6$						
Τ	$[m^2/s]$	$1.2 \cdot 10^{-4}$	$1.0 \cdot 10^{-4}$					
Κ	[m/s]	$8.2 \cdot 10^{-7}$	$6.6 \cdot 10^{-7}$					
S	$[-]$	-	$1.6 \cdot 10^{-4}$					
ω	$[ - ]$	0.07	0.21					
λ	$[ - ]$	$8.0 - 10^{-7}$	$1.9.10^{-6}$					

**Table 5-2. Derived aquifer parameters (nomenclature table is in Appendix 7 for explanations).**

#### *HLX34*

The diagnostic log-log plots revealed that flow regime were close to radial flow according to the Theis model but that possibly there is additional flow component . The best match of both drawdown, recovery as well as history data are with a pseudo steady state double porosity model. Consistent values were derived of transmissivity and storage coefficient. The drawdown phase approaches steady state conditions. Derived aquifer parameters are given in Table 5-2.

#### *HLX35*

The diagnostic log-log plots show reveal an initial radial flow followed by a delayed yield type of behaviour. Conceptually both a double porosity and a radial composite flow regime are plausible in this hydrogeological environment. Indeed both model could be matched quite well to the drawdown and recovery data but better consitencies between drawdown and recovery were achived with the double porosity concept. This is consitent with the fractured rock and also conforms to the result in HLX34. Consistent values were derived of transmissivity. The storage coefficient derived from the interference test with HLX34 was utilised as input to the analysis of the test in HLX35,  $S = 1.5 \cdot 10^{-4}$ . Derived aquifer parameters are given in Table 5-2.

## <span id="page-21-0"></span>**5.2 Tracer test**

#### **5.2.1 Dilution measurements**

Tracer dilution measurements were performed in boreholes SSM000037, SSM000222 and SSM000223 both before and during pumping in HLX35. The results are presented in Table 5-3 and the dilution graphs are shown in Figure 5-1.







*Figure 5-1. Tracer dilution graphs (Logarithm of concentration versus time) for the measured boreholes SSM000037, SSM000222 and SSM000223 including straight-line fits. Note that the axis scales differ between the plots.*

<span id="page-22-0"></span>Notable is that the gradient seems to be reversed or partly reversed in all three soil wells when the pumping started in HLX35. As seen in the dilution graph (Figure 5-1), the tracer concentration in SSM037 increased after the pumping started, implying that tracer that earlier had left the borehole was withdrawn when the gradient and flow direction was reversed due to pumping in HLX35. In SSM000222 and SSM000223 the flow was decreased during the pumping phase, which also indicates a change in gradient/flow direction. In SSM000223 the dilution was very high during natural conditions and when the pumping started there was not tracer enough left in the borehole for detection. Therefore, the groundwater flow during stressed conditions in SSM000223 is determined from the dilution of the tracer injected in the tracer test.

#### **5.2.2 Tracer test**

Tracer injections were performed in boreholes SSM000222 and SSM000223. In Table 5-4 tracer injection data is presented together with the distance between injection borehole and pumping borehole HLX35. The distances are calculated at half the length of each borehole.

HLX35 was pumped with a withdrawal rate of 104 L/min and the water was continuously sampled for tracer breakthrough.

Tracer breakthrough in HLX35 was detected from the injection of Rhodamine WT in borehole SSM000223, see Figure 5-2. Only the samples taken by the automatic 24-valve sampler showed tracer breakthrough. No tracer breakthrough was detected from the injection of Uranine in borehole SSM000222.





<sup>1)</sup> Concentration of the solution utilised prior to injection in the well.

 $2)$  Initial concentration in the soil well, 2 hours after injection start.

<sup>3)</sup> Injected mass corrected for tracer withdrawal during injection exchange procedure and sampling.



*Figure 5-2. Measured data and model simulations of tracer breakthrough (concentration versus time) in HLX35 from the injection of Rhodamine WT in SSM000223.*

Tracer mass recovery for Rhodamine WT was 7% when sampling was stopped 635 hours after the injection.

The breakthrough curve was evaluated using the one-dimensional advection-dispersion model described in Section 4.4. The best-fit run is shown in Figure 5-2 (left) and the parameters determined from the model run are presented in Table 5-5. The regression statistics show high standard errors which could be expected due to the scattered breakthrough data. The parameters obtained were then used to simulate the breakthrough curve ahead, Figure 5-2 (right). After 8,000 hours the recovery would then be 24%.

**Table 5-5. Evaluated parameters using PAREST (one-dimensional advection-dispersion** model) for the flow path SSM000223 → HLX35. Values within brackets are standard errors **in percent.**

Injection borehole	Tracer	Distance (m)	$v$ (m/s)	$t_m(h)$	$D/v$ (m)	F
SSM000223	Rhodamine WT	72 (ground distance)	$2.06 \cdot 10^{-5}$ (49)	970	69 (17)	$6.56 \cdot 10^{-5}$ (55)
		102 (between inflow pont)	$3.09 \cdot 10^{-5}$ (45)	916	89 (17)	$6.15 \cdot 10^{-5}$ (51)
		132 (along contact soil/rock to DZ)	$3.15 \cdot 10^{-5}$ (64)	1.165	159 (17)	$8.06 \cdot 10^{-5}$ (71)

# <span id="page-24-0"></span>**6 Summary and discussions**

A field investigation was undertaken in order to study the hydraulic contact between rock and soil aquifer by means of pressure interference and tracer testing.

A borehole drilled into a deformation zone was pumped while three soil wells and one borehole were used for response observations. Tracer testing was performed with conservative tracers only, in two steps. Firstly, dilution measurements were done in the soil wells under unstressed conditions in order to obtain natural groundwater flow velocities and secondly a forced gradient test was done where tracer was injected in one of the soil wells and recovered in the pumped borehole.

A hydraulic contact between rock and soil could be established through these works by means of both diffusive and advective transport.

It is believed that the transport took place through the deformation zone NS059A which essentially appear to be in direct contact with the soil aquifer at SSM000223. The simultaneous apparent lack of response in the soil wells SSM000037 and SSM00022 indicates a very local influence by the pumping in the rock to the soil, at least with respect to the durations, distances and disturbances applied in this test. A hydraulically active rock/soil contact is basically governed by the presence of a high permeability feature in the rock, such as the deformations zone.

It could also be established that the amount of water level fluctuation in the soil at SSM000223 was of the same amount as fluctuations caused by natural recharge/discharge processes.

The works also allowed a quantification of aquifer properties and solute transport parameters. In summary, the main observations emanating from this study are as follow:

- Hydraulic pressure interference from HLX35 was observed at HLX34 with a response index  $r_s^2/dt_L = 10$  and at SSM000223 with a response index  $r_s^2/dt_L = 0.07$ . No interference was observed at SSM000037 nor SSM000222.
- Double porosity flowregime was observed in the rock supporting the fracture zone- intact rock concept and a homogeneous radial flow in the soil aquifer.
- Obtained T-values are  $1 \cdot 10^{-4}$  m<sup>2</sup>/s for the deformation zone and  $4 \cdot 10^{-3}$  m<sup>2</sup>/s for the soil aquifer.
- Obtained S-values are  $1 \cdot 10^{-4}$  for the deformation zone and  $8 \cdot 10^{-3}$  for the soil aquifer.
- Water levels in the soil were much influenced by recharge events due to precipitation.
- A relatively high natural groundwater flowrate of 107 mL/min was measured at SSM000223 which is situated at the foot of the forested slope bordering the arable land and much lower rates in the wells in the middle of the field, close to the stream. Hence, the natural flow rates are very much topgraphically controlled, and not so much by the stratigraphy.
- The hydraulic gradient seemed to be reversed or partly reversed in the soil wells when pumping started in HLX35.
- Tracer breakthrough was detected in HLX35 from the injection of Rhodamine WT in SSM000223.
- No tracer breakthrough was detected from the injection of Uranine in SSM000222.
- Recovery (Rhodamine WT) was 7% after 635 hours of sampling.
- From model simulation of the breakthrough curve (Rhodamine WT) a mean travel time of 970 hours and a dispersivity of 69 m were obtained. However, the standard errors were high due to the scattered breakthrough data. By simulating the breakthrough curve ahead a recovery of 24% was reached after 8,000 hours.

# <span id="page-25-0"></span>**References**

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## <span id="page-26-0"></span>**Response hydrographs for HLX35, SSM000037, SSM000222 and SSM000223**



Response wells SSM37, SSM222 and SSM223 on pumping in HLX35



# <span id="page-27-0"></span>**Geological cross section of the tested site**



<span id="page-28-0"></span>Appendix 3 **Appendix 3**

# Stratigraphy and lithology **Stratigraphy and lithology**













<span id="page-31-0"></span>**Response hydrographs and precipitation for the stream Ekerumsån and for soil wells SSM000223, SSM000222 and SSM000037**







<span id="page-33-0"></span>**Air temperature and precipitation measured at Äspö meteorological station situated 4 km due east of the test site**

## <span id="page-34-0"></span>**Analysis of pumping and interference tests in SSM000223, HLX34 and HLX35**

For each of these boreholes, analysis and simulations are done for the drawdown phase, the recovery phase and the complete flow/pressure history.





Ecrin v4.02.03 SSM223obs\_masl









Ecrin v4.02.03 SSM223obs\_masl









Ecrin v4.02.03 HLX34obs\_masl









Ecrin v4.02.03 HLX34obs\_masl













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![](_page_56_Figure_0.jpeg)

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![](_page_58_Figure_0.jpeg)

# **Appendix 7**

## <span id="page-59-0"></span>**Nomenclature**

#### **Yellow marked parameters are used in SICADA while the other appear only in the P-report.**

![](_page_59_Picture_357.jpeg)

![](_page_60_Picture_346.jpeg)

![](_page_61_Picture_366.jpeg)

![](_page_62_Picture_190.jpeg)

![](_page_63_Figure_1.jpeg)

# <span id="page-63-0"></span>**Map of site investigation subarea Laxemar**